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Universitat Politècnica de Catalunya

Escola Tècnica Superior d'Enginyers de Camins, Canals i Ports

MASTER EN INGENIERÍA ESTRUCTURAL Y DE LA CONSTRUCCIÓN

Asignatura:

# **COMPUTATIONAL STRUCTURAL MECHANICS AND DYNAMICS**

**Assignment 2**

**On “FEM Modelling: Introduction”**

**And “Variational Formulation”**

**By**

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## Assignment 2.1:

On “FEM Modelling: Introduction”:

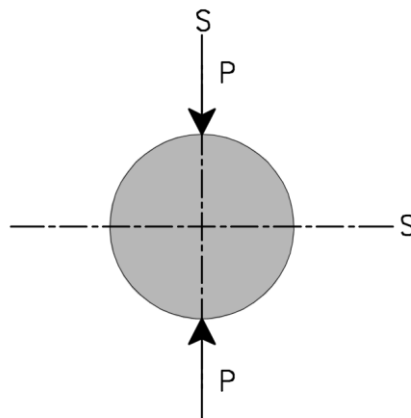
### Exercise 1

Identify the symmetry and antisymmetry lines in the two-dimensional problems illustrated in the figure. They are:

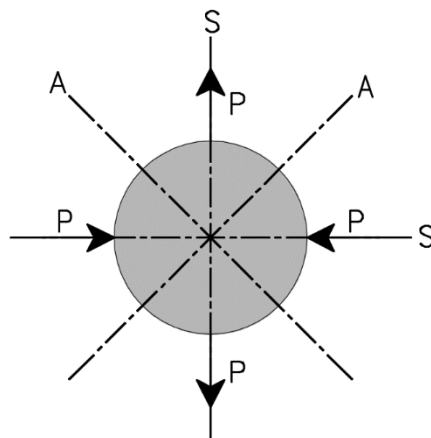
- (a) a circular disk under two diametrically opposite point forces (the famous “Brazilian test” for concrete)
  - (b) the same disk under two diametrically opposite force pairs
  - (c) a clamped semiannulus under a force pair oriented as shown
  - (d) a stretched rectangular plate with a central circular hole.
  - (e) and (f) are half-planes under concentrated loads.
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In the figures the solutions. S: symmetry line, A: antisymmetric line

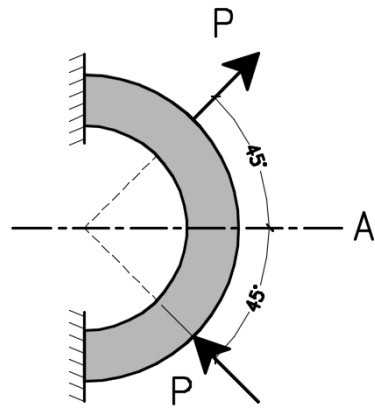
(a)



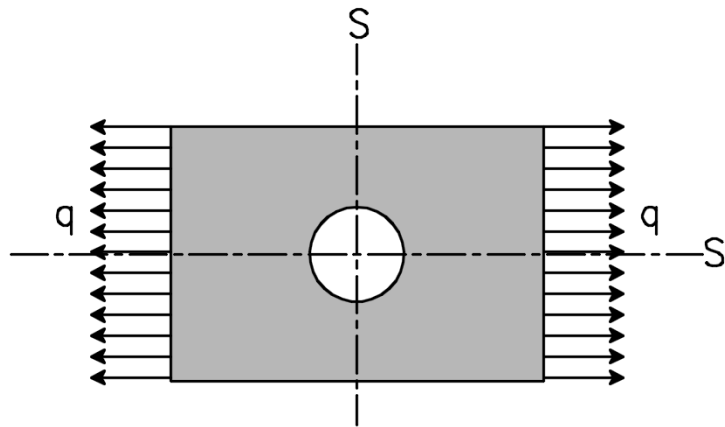
(b)



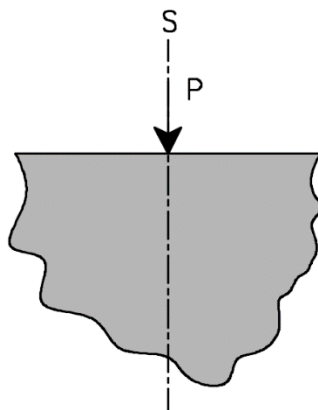
(c)



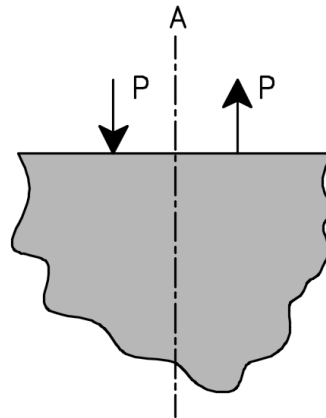
(d)



(e)



(f)

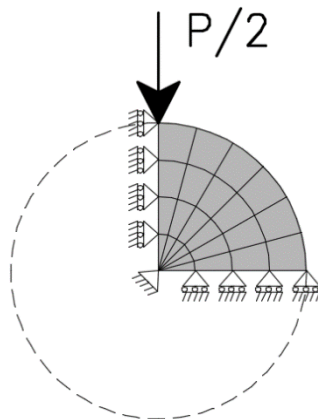


### Exercise 2

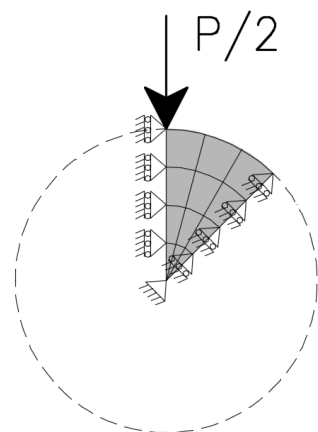
Having identified those symmetry/antisymmetry lines, state whether it is possible to cut the complete structure to one half or one quarter before laying out a finite element mesh. Then draw a coarse FE mesh indicating, with rollers or fixed supports, which kind of displacement BCs you would specify on the symmetry or antisymmetry lines.

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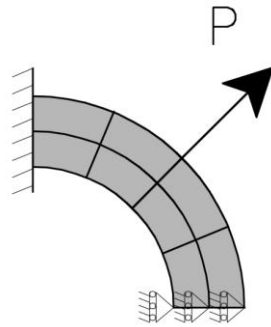
(a) On double symmetric axes



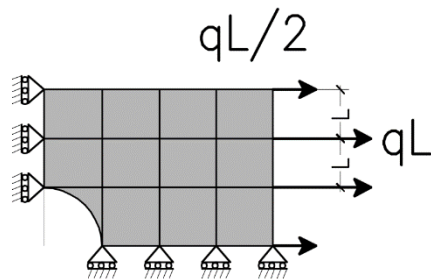
(b) Using one symmetric and one anti-symmetric axes



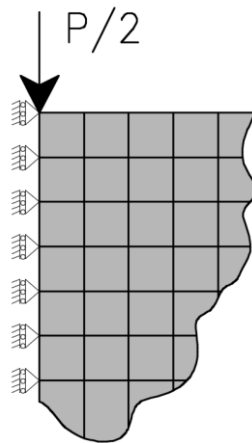
(c) On single anti-symmetric axes



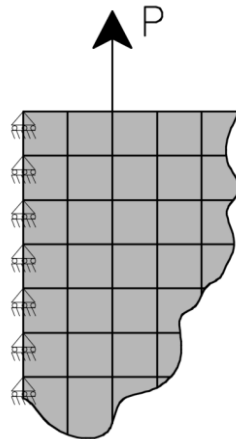
(d) On double symmetric axes



(e) On single symmetric load distribution



(f) On single anti-symmetric load distribution



## Assignment 2.2:

On "FEM Modelling: Introduction":

### Exercise 1

The plate structure shown in the figure is loaded and deforms in the plane of the paper. The applied load at D and the supports at I and N extend over a fairly narrow area. List what you think are the likely "trouble spots" that would require a locally finer finite element mesh to capture high stress gradients. Identify those spots by its letter and a reason.

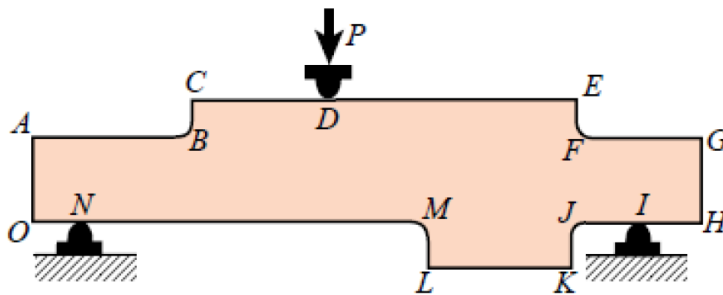


Figure 2.2.- Inplane bent plate

Trouble spots:

- Points D, N and I: there are concentrated loads on that points.
- Points B, F, M and J: abrupt change of section of the piece.

## Assignment 2.3:

On "Variational Formulation":

### Exercise 1

A tapered bar element of length  $l$  and areas  $A_i$  and  $A_j$  with  $A$  interpolated as

$$A = A_i(1 - \xi) + A_j\xi$$

and constant density  $\rho$  rotates on a plane at uniform angular velocity  $\omega$  (rad/sec) about node  $i$ .

Taking axis  $x$  along the rotating bar with origin at node  $i$ , the centrifugal axial force is  $q(x) = \rho A \omega^2 x$  along the length in which  $x$  is the longitudinal coordinate  $x = x^e$ .

Find the consistent node forces as functions of  $\rho$ ,  $A_i$ ,  $A_j$ ,  $\omega$  and  $l$ , and specialize the result to the prismatic bar  $A = A_i = A_j$ .

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First step is change variables on  $q(x)$

$$\begin{aligned} x^e &= x - x_i \\ \xi &= \frac{x - x_i}{l} \rightarrow x^e = \xi l \end{aligned}$$

Replacing on  $q(x)$  equation

$$q(x^e) \Rightarrow q(\xi) = \rho A(\xi) \omega^2 \xi l$$

So, using the form equations in slides, the equation for consistent node forces is

$$\mathbf{f}_{ext} = \int_0^1 q \begin{bmatrix} 1 - \xi \\ \xi \end{bmatrix} l d\xi = \int_0^1 \rho (A_i(1 - \xi) + A_j\xi) \omega^2 \xi \begin{bmatrix} 1 - \xi \\ \xi \end{bmatrix} l^2 d\xi$$

Solving for each component

$$f_{ext_i} = \int_0^1 \rho (A_i(1 - \xi) + A_j\xi) \omega^2 \xi (1 - \xi) l^2 d\xi = \frac{1}{12} \rho \omega^2 l^2 (A_i + A_j)$$

$$f_{ext_j} = \int_0^1 \rho (A_i(1 - \xi) + A_j\xi) \omega^2 \xi^2 l^2 d\xi = \frac{1}{12} \rho \omega^2 l^2 (A_i + 3A_j)$$

$$\mathbf{f}_{ext} = \frac{1}{12} \rho \omega^2 l^2 \begin{pmatrix} A_i + A_j \\ A_i + 3A_j \end{pmatrix}$$

If  $A = A_i = A_j$

$$\mathbf{f}_{ext} = \frac{1}{12} \rho \omega^2 l^2 \begin{pmatrix} 2A \\ 4A \end{pmatrix} = \frac{1}{6} \rho \omega^2 l^2 \begin{pmatrix} A \\ 2A \end{pmatrix}$$