

ASSIGNMENT 3
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ASSIGNMENT 3.1

a) We want E^* in terms of E , and ν^* in terms of ν , where E^* is a fictitious modulus in the plane stress relation and E in the plane strain.

Also ν^* is a Fictitious Poisson's ratio in the plane stress relation and ν in the plane strain relation.

So, substituting E by E^* and ν by ν^* in the plane stress relation we get:

$$\begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{bmatrix} = \frac{E^*}{1 - \nu^{*2}} \begin{bmatrix} 1 & \nu^* & 0 \\ \nu^* & 1 & 0 \\ 0 & 0 & \frac{1 - \nu^*}{2} \end{bmatrix} \begin{bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ 2\epsilon_{xy} \end{bmatrix}$$

Comparing elements 11 of each expressions and 12 we have:

$$\left. \begin{aligned} \frac{E^*}{1 - \nu^{*2}} &= \frac{E(1 - \nu)}{(1 + \nu)(1 - 2\nu)} \\ \frac{E^*}{(1 - \nu^{*2})} \nu^* &= \frac{E(1 - \nu)\nu}{(1 + \nu)(1 - 2\nu)(1 - \nu)} \end{aligned} \right\}$$

$$\frac{E(1-\nu)}{(1+\nu)(1-2\nu)} \nu^* = \frac{E\nu}{(1+\nu)(1-2\nu)}$$

$$\boxed{\nu^* = \frac{\nu}{1-\nu}}$$

$$\frac{E^*}{(1-\nu^{*2})} = \frac{E(1-\nu)}{(1+\nu)(1-2\nu)}$$

$$\downarrow \nu^* = \frac{\nu}{1-\nu}$$

$$\frac{E^*}{1 - \left(\frac{\nu}{1-\nu}\right)^2} = \frac{E(1-\nu)}{(1+\nu)(1-2\nu)}$$

$$\frac{E^*}{1 + \cancel{2\nu} - 2\nu \cancel{\rightarrow} \nu^2} = \frac{E^*}{(1-\nu)^2} = E^* \frac{(1-\nu)^2}{(1-2\nu)} = \frac{E(1-\nu)}{(1+\nu)(1-2\nu)}$$

$$E^* = \frac{E}{(1+\nu)(1-2\nu)}$$

$$\boxed{E^* = \frac{E}{1-\nu^2}}$$

We can do the inverse process: go from plane stress to plane strain by replacing a fictitious modulus and Poisson's ratio in the plane strain constitutive matrix. Let's do it:
 We call E^* to the fictitious modulus and ν^* to the Poisson's ratio.

And we do it again with the elements 11 and 22:

$$\left. \begin{aligned} \frac{E^* (1 - \nu^{*2})}{(1 + \nu^*)(1 - 2\nu^*)} &= \frac{E}{1 - \nu^2} \\ \frac{E^* (1 - \nu^{*2})}{(1 + \nu^*)(1 - 2\nu^*)} \frac{\nu^*}{2 - \nu^*} &= \frac{E \nu}{1 - \nu^2} \end{aligned} \right\}$$

$$\begin{aligned} \frac{E}{1 - \nu^2} \frac{\nu^*}{1 - \nu^*} &= \frac{E \nu}{1 - \nu^2} \\ \frac{\nu^*}{1 - \nu^*} &= \frac{\nu (1 - \nu^2)}{1 - \nu^2} \\ \nu^* (1 + \nu) &= \nu \\ \nu^* &= \frac{\nu}{1 + \nu} \end{aligned}$$

And:

$$\frac{E^* (1 - \nu^*)}{(1 + \nu^*) (1 - 2\nu^*)} = \frac{E}{1 - \nu^2}$$

$$\nu^* = \frac{\nu}{1 + \nu}$$

$$\frac{E^* \left(1 - \frac{\nu}{1 + \nu}\right)}{\left(1 + \frac{\nu}{1 + \nu}\right) \left(1 - 2\frac{\nu}{1 + \nu}\right)} = \frac{E^* \frac{1}{1 + \nu}}{\left(\frac{1 + 2\nu}{1 + \nu}\right) \left(\frac{1 - \nu}{1 + \nu}\right)} =$$

$$= E^* \frac{1}{(1 + \nu)} \frac{(1 + \nu)(1 + \nu)}{(1 + 2\nu)(1 - \nu)} = \frac{E}{(1 + \nu)(1 - \nu)}$$

$$E^* = \frac{E}{(1 + \nu)(1 - \nu)} \frac{(1 + 2\nu)(1 - \nu)}{(1 + \nu)} = \frac{1 + 2\nu}{(1 + \nu)^2} E$$

$$E^* = \frac{1 + 2\nu}{(1 + \nu)^2} E$$

b)

$$\begin{aligned}
 U &= \frac{1}{2} [\sigma_{xx} e_{xx} + \sigma_{yy} e_{yy} + \sigma_{xy} e_{xy} + \sigma_{yx} e_{yx}] = \\
 &= \frac{1}{2} [\sigma_{xx} e_{xx} + \sigma_{yy} e_{yy} + 2\sigma_{xy} e_{xy}] = \\
 &= \frac{1}{2} (e_{xx} \ e_{yy} \ 2e_{xy}) \begin{pmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{pmatrix} = \frac{1}{2} e^T \sigma = (*)
 \end{aligned}$$

In terms of strains:

$$\begin{pmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{pmatrix} = \underbrace{\frac{E(1-\nu)}{(1+\nu)(1-2\nu)}}_E \begin{pmatrix} 1 & \frac{\nu}{1-\nu} & 0 \\ \frac{\nu}{1-\nu} & 1 & 0 \\ 0 & 0 & \frac{1-2\nu}{2(1-\nu)} \end{pmatrix} \begin{pmatrix} e_{xx} \\ e_{yy} \\ 2e_{xy} \end{pmatrix} = E e$$

$$(*) = \frac{1}{2} e^T E e$$

$$\boxed{U = \frac{1}{2} e^T E e}$$

The internal energy can be written in terms of stresses:

$$U = \frac{1}{2} \sigma^T C \sigma, \text{ where } C = E^{-1}$$

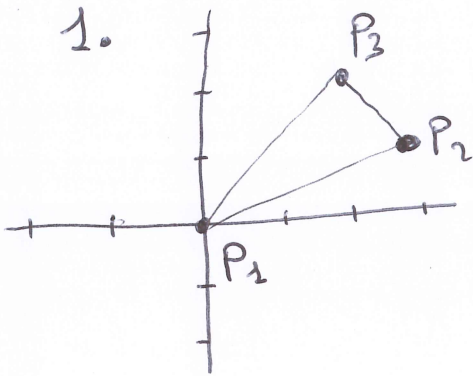
$$\begin{aligned}
 U &= \frac{1}{2} (\sigma_{xx} e_{xx} + \sigma_{yy} e_{yy} + \sigma_{xy} e_{xy} + \sigma_{yx} e_{yx}) = \\
 &= \frac{1}{2} (\sigma_{xx} e_{xx} + \sigma_{yy} e_{yy} + 2\sigma_{xy} e_{xy}) = \\
 &= \frac{1}{2} (\sigma_{xx} \ \sigma_{yy} \ \sigma_{xy}) \begin{pmatrix} e_{xx} \\ e_{yy} \\ 2e_{xy} \end{pmatrix} = \frac{1}{2} \sigma^T \begin{pmatrix} e_{xx} \\ e_{yy} \\ 2e_{xy} \end{pmatrix} = \\
 &= \frac{1}{2} \sigma^T \underbrace{E^{-1}}_I E \begin{pmatrix} e_{xx} \\ e_{yy} \\ 2e_{xy} \end{pmatrix} = \frac{1}{2} \sigma^T \underbrace{E^{-1} E}_I \begin{pmatrix} e_{xx} \\ e_{yy} \\ 2e_{xy} \end{pmatrix} =
 \end{aligned}$$

$$E = \begin{pmatrix} \frac{1}{1-\nu^2} & \frac{\nu}{1-\nu^2} & 0 \\ \frac{\nu}{1-\nu^2} & \frac{1}{1-\nu^2} & 0 \\ 0 & 0 & \frac{1-\nu}{2(1-\nu^2)} \end{pmatrix} \text{ in plane stress expression}$$

$$\frac{E}{2(1-\nu^2)} \begin{pmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{pmatrix} \begin{pmatrix} e_{xx} \\ e_{yy} \\ 2e_{xy} \end{pmatrix} = \begin{pmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{pmatrix} = \sigma$$

$$\boxed{U = \frac{1}{2} \sigma^T \underbrace{E^{-1}}_C \underbrace{E}_\sigma e = \frac{1}{2} \sigma^T C \sigma}$$

ASSIGNMENT 3.2.



$$\begin{array}{ll}
 P_1(0,0) & P_1P_2 = \sqrt{10} \\
 P_2(3,1) & P_1P_3 = \sqrt{8} \\
 P_3(2,2) & P_2P_3 = \sqrt{2}
 \end{array}$$

$h=1$, we suppose h is constant. So we must use:

$$k_e = \frac{h}{4A} \begin{pmatrix} y_{23} & 0 & x_{32} \\ 0 & x_{32} & y_{23} \\ y_{31} & 0 & x_{13} \\ 0 & x_{13} & y_{31} \\ y_{12} & 0 & x_{21} \\ 0 & x_{21} & y_{12} \end{pmatrix} \begin{pmatrix} E_{11} & E_{12} & E_{13} \\ E_{21} & E_{22} & E_{23} \\ E_{31} & E_{32} & E_{33} \end{pmatrix} \begin{pmatrix} y_{23} & 0 & y_{31} & 0 & y_{12} & 0 \\ 0 & x_{32} & 0 & x_{13} & 0 & x_{21} \\ x_{32} & y_{23} & x_{13} & y_{31} & x_{21} & y_{12} \end{pmatrix}$$

A is the triangle area. We compute it by Heron Formula.
 $s = \text{semiperimeter} = \frac{1}{2} (P_1P_2 + P_1P_3 + P_2P_3) = \frac{\sqrt{2}}{2} (\sqrt{5} + 3)$

$$A = \sqrt{s(s - P_1P_2)(s - P_1P_3)(s - P_2P_3)}$$

$$A \approx 2$$

Taking into account that:

$$y_{jk} = y_j - y_k$$

$$x_{jk} = x_j - x_k$$

$$\left. \begin{array}{l} y_{23} = y_2 - y_3 = -1 \\ y_{31} = y_3 - y_1 = 2 \\ y_{12} = y_1 - y_2 = -1 \end{array} \right\} \begin{array}{l} x_{21} = 3 \\ x_{32} = -1 \\ x_{13} = -2 \end{array}$$

$$K^e = \frac{1}{8} \times 25 \begin{pmatrix} -1 & 0 & -1 \\ 0 & -1 & 1 \\ 2 & 0 & -2 \\ 0 & -2 & 2 \\ -1 & 0 & 3 \\ 0 & 3 & -1 \end{pmatrix} \begin{pmatrix} 4 & 1 & 0 \\ 1 & 4 & 0 \\ 0 & 0 & 2 \end{pmatrix} \begin{pmatrix} -1 & 0 & 2 & 0 & -1 & 0 \\ 0 & -1 & 0 & -2 & 0 & 3 \\ -1 & -1 & -2 & 2 & 0 & -1 \end{pmatrix} =$$

$$= \frac{25}{8} \begin{pmatrix} 6 & 3 & -4 & -2 & -2 & -1 \\ 3 & 6 & 2 & 4 & -5 & 10 \\ -4 & 2 & 24 & -12 & -20 & 10 \\ -2 & 4 & -20 & 14 & 14 & -28 \\ -2 & -5 & 14 & 22 & 22 & -9 \\ -1 & -10 & -28 & -18 & -9 & 38 \end{pmatrix}$$

So:

$$K_e = \begin{pmatrix} 18.7500 & 9.3750 & -12.5000 & -6.2500 & -6.2500 & -3.1250 \\ 9.3750 & 18.7500 & 6.2500 & 12.5000 & -15.6250 & -31.2500 \\ -12.5000 & 6.2500 & 75.0000 & -37.5000 & -62.5000 & 31.2500 \\ -6.2500 & 12.5000 & -37.5000 & 75.0000 & 43.7500 & -87.5000 \\ -6.2500 & -15.6250 & -62.5000 & 43.7500 & 68.7500 & -28.1250 \\ -3.1250 & -31.2500 & 31.2500 & -87.5000 & -28.1250 & 118.7500 \end{pmatrix}$$

So $k_{11} = 18,75$ $k_{21} = 9,375$ \dots
 $k_{21} = 9,375$ \vdots
 \vdots
 \vdots
 $\dots \dots \dots k_{66} = 118,75$

$$2. \quad (r_1 + r_3 + r_5) = \begin{pmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{pmatrix}$$

$r_i = \text{row } i$
 $c_i = \text{column } i$

$$r_2 + r_4 + r_6 = 0$$

$$c_1 + c_3 + c_5 = 0$$

$$c_2 + c_4 + c_6 = 0$$

$\sum r_i = \sum c_i = 0$
 because K_e is symmetric

$$K_e u = F$$

$$\begin{pmatrix} F_{x1} \\ F_{y1} \\ F_{x2} \\ F_{y2} \\ F_{x3} \\ F_{y3} \end{pmatrix} = K_e \begin{pmatrix} u_{x1} \\ u_{y1} \\ u_{x2} \\ u_{y2} \\ u_{x3} \\ u_{y3} \end{pmatrix}$$

$$F_{x1} = k_{11} u_{x1} + \dots + k_{16} u_{y3}$$

$$F_{y1} = k_{21} u_{x1} + \dots + k_{26} u_{y3}$$

$$F_{x3} = k_{31} u_{x1} + \dots$$

\vdots

$$F_{y3} = \dots$$

columns

$$k_{11} + k_{12} + k_{15} = 0$$

$$k_{21} + k_{23} + k_{25} = 0$$

$$k_{31} + k_{33} + k_{35} = 0$$

$$k_{41} + k_{43} + k_{45} = 0$$

$$k_{51} + k_{53} + k_{55} = 0$$

$$k_{61} + k_{63} + k_{65} = 0$$

$$\rightarrow k_{12} + k_{14} + k_{16} = 0$$

$$k_{22} + k_{44} + k_{66} = 0$$

$$k_{32} + k_{34} + k_{36} = 0$$

$$k_{42} + k_{44} + k_{46} = 0$$

$$k_{52} + k_{54} + k_{56} = 0$$

$$k_{62} + k_{64} + k_{66} = 0$$

rows

$$\rightarrow k_{11} + k_{31} + k_{51} = 0$$

$$\rightarrow k_{13} + k_{33} + k_{53} = 0$$

$$\rightarrow k_{15} + k_{35} + k_{55} = 0$$

$$k_{12} + k_{32} + k_{52} = 0$$

$$k_{14} + k_{34} + k_{54} = 0$$

$$\Rightarrow k_{16} + k_{36} + k_{56} = 0$$

$$k_{22} + k_{44} + k_{66} = 0$$

$$k_{23} + k_{43} + k_{63} = 0$$

$$k_{24} + k_{44} + k_{64} = 0$$

$$k_{25} + k_{45} + k_{65} = 0$$

$$k_{26} + k_{46} + k_{66} = 0$$

Sum of forces on x coordinate

$$F_{x1} + F_{x3} + F_{x5} = k_{11}u_{x1} + k_{12}u_{x2} + k_{13}u_{x3} + k_{14}u_{x2} + k_{15}u_{x3} +$$

$$+ k_{16}u_{x3} + k_{31}u_{x1} + k_{32}u_{x2} + k_{33}u_{x2} + k_{34}u_{x1} + k_{35}u_{x3} +$$

$$+ k_{36}u_{x3} + k_{51}u_{x1} + k_{52}u_{x2} + k_{53}u_{x2} + k_{54}u_{x1} + k_{55}u_{x3} +$$

$$+ k_{56}u_{x3} = \underbrace{(k_{11} + k_{31} + k_{51})}_{=0} u_{x1} + \underbrace{(k_{12} + k_{14} + k_{16})}_{=0} u_{x2} +$$

$$+ \underbrace{(k_{13} + k_{33} + k_{53})}_{=0} u_{x2} + \underbrace{(k_{14} + k_{34} + k_{54})}_{=0} u_{x2} +$$

$$+ \underbrace{(k_{15} + k_{35} + k_{55})}_{=0} u_{x3} + \underbrace{(k_{16} + k_{36} + k_{56})}_{=0} u_{x3}$$

We can see that $f_{x_1} + f_{x_2} + f_{x_3} = 0$

(because of the \rightarrow selected)

Similarly we will find that $f_{y_1} + f_{y_2} + f_{y_3} = 0$

$$\text{So } \sum f_{x_i} = 0$$

$$\text{and } \sum f_{y_i} = 0$$

The system is in equilibrium of forces. 15

the reason of $\left\{ \begin{array}{l} \sum v_i = 0 \\ \sum c_i = 0 \end{array} \right.$